PWT™ I-Joist Series PWT

PR-L262

Revised January 26, 2024

Products: PWT™ I-joist Series

PWT, 1850 Park Lane, Burlington, Washington 98233

(888) 707-2285 www.pwtewp.com

1. Basis of the product report:

- 2021, 2018, 2015, and 2012 International Building Code (IBC): Sections 104.11 Alternative materials and 2303.1.2 Prefabricated wood I-joists
- 2021, 2018, and 2015 International Residential Code (IRC): Sections 104.11 Alternative materials, R502.1.2 Prefabricated wood I-joists, and R802.1.8 (2021 and 2018 IRC only) Prefabricated wood I-joists.
- 2012 International Residential Code (IRC): Sections R104.11 Alternative materials and R502.1.4 Prefabricated wood I-joists
- ASTM D5055-16, D5055-13e1, D5055-13, and D5055-09 recognized in the 2021 IBC and IRC, 2018 IBC and IRC, 2015 IBC and IRC, and 2012 IBC and IRC, respectively
- APA PRI-400, Performance Standard for Residential I-joist
- 2021, 2015, and 2008 ANSI/AWC Special Design Provisions for Wind and Seismic (SDPWS) recognized in the 2021, 2018 and 2015, and 2012 IBC, respectively
- APA Reports T2000P-2, T2000P-5, T2001P-25A, T2002P-41, T2003P-58, T2003P-68, T2008P-83, T2008P-84, T2009P-49, T2014P-42, T2014P-43, T2015P-23, T2016P-40, T2021P-10, T2021P-45, T2021P-46, and T2022P-09, and other qualification data

Product description:

PWT[™] I-joist series, PWI, covered by this report, as described in Table 1, are made with laminated veneer lumber (LVL) flanges and OSB webs in accordance with the in-plant manufacturing standard and quality manual approved by APA.

Design properties:

Tables 2a and 2b list the design properties for PWI joists. PWI-47 joists shall be permitted for use in wood-frame diaphragm applications when designed in accordance with Tables 4.2A and 4.2C of the AWC SDPWS as follows:

- a) The nominal framing width must be limited to 2 inches in the SDPWS tables when the following conditions are met:
 - 1) Nail spacing must be 4 inches or 6 inches on center, and
 - 2) When the nail spacing is 4 inches on center at diaphragm boundaries, adjacent nails within a row at diaphragm boundaries must be offset (staggered) at least 1/2 inch.
- b) The nominal framing width must be limited to 3 inches in the SDPWS tables when the following conditions are met:
 - 1) Nail spacing must be 4 inches or 6 inches on center,
 - 2) When the nail spacing is 4 inches on center at diaphragm boundaries, adjacent nails within a row at diaphragm boundaries must be offset (staggered) at least 1/2 inch,
 - 3) I-joist floor assembly must be constructed with minimum 1-1/2-inch thick LVL rim boards in the direction perpendicular the I-joist framing, bearing on double 2x6 top wall plates or 2x sill plates, and
 - 4) I-joists must be installed with construction adhesive meeting ASTM D3498 1/8" P/O class between the sheathing and I-joist.

The allowable spans for PWI joists shall be in accordance with the recommendations provided by the manufacturer (www.pwtewp.com).

4. Product installation:

PWI joists shall be installed in accordance with the recommendations provided by the manufacturer. Table 3 shows web stiffener information. Permissible web holes and cantilever reinforcements shall be in accordance with the recommendations provided by the manufacturer.

Fire-rated assemblies:

Fire-rated assemblies shall be constructed in accordance with the recommendations provided by the manufacturer, APA Product Report PR-S262, or APA Design/Construction Guide: *Fire-Rated Systems*, Form W305 (www.apawood.org/resource-library).

Limitations:

- a) PWI joists shall be designed in accordance with the code using the design properties specified in this report.
- b) PWI joists are limited to dry service conditions where the average equilibrium moisture content of sawn lumber is less than 16%.
- c) PWI joists are produced at the PWT manufacturing plant located in Burlington, Washington, under a quality assurance program audited by APA.
- d) This report is subject to re-examination in one year.

Identification:

PWI joists are sold under the PWT and various private-label brands. Regardless of the brand applied, all PWI joists are identified by a label bearing the manufacturer's name (PWT) and/or trademark, the APA assigned plant number (1048), the I-joist depth and series, the APA logo, the report number PR-L262, and a means of identifying the date of manufacture.

Table 1. Description of PWI Joists(a)

		Flanges			Web		
Joist Series	Joist Depths (in.)	Motorial	Dimension		Motorial	Thickness	
001100	()	Material	Depth (in.)	Width (in.)	Material	(in.)	
PWI-20	7-7/8 – 14	LVL	1-3/8	1-3/4	OSB	3/8	
PWI-30	9-1/2 – 11-7/8	LVL	1-1/2	1-1/2	OSB	3/8	
PWI-40	9-1/4 — 16	LVL	1-3/8	2-5/16	OSB	3/8	
PWI-45	9-1/2 - 16	LVL	1-3/8	2-1/16	OSB	3/8	
PWI-47	7-7/8 – 20	LVL	1-1/8	2-5/16	OSB	3/8	
PWI-50	9-1/2 – 16	LVL	1-1/2	1-3/4	OSB	3/8	
PWI-60	9-1/4 – 16	LVL	1-3/8	2-5/16	OSB	3/8	
PWI-70	9-1/2 – 20	LVL	1-1/2	2-5/16	OSB	3/8	
PWI-77	9-1/2 – 24	LVL	1-1/2	2-5/16	OSB	7/16	
PWI-77w	9-1/2 – 24	LVL	1-1/2	2-1/2	OSB	7/16	
PWI-90	9-1/2 – 24	LVL	1-1/2	3-1/2	OSB	7/16	

⁽a) Referenced dimensions are nominal. Tolerances are as specified in the in-plant quality manual.

Table 2a. Design Properties (Allowable Stress Design) for PWI Joists^(a)

Table Za. De				n) for PWI Jois		Uniform
Joist Series	Joist Depth	EI ^(b)	k ^(c)	M ^(d)	V(e)	Vertical Load(f)
	(in.)	(10 ⁶ lbf-in. ²)	(10 ⁶ lbf)	(ft-lbf)	(lbf)	(plf)
	7-7/8	113	4.10	2340	1170	2000
DW/I 00	9-1/2	157	4.94	2520	1330	2000
PWI-20	11-7/8	267	6.18	3265	1705	2000
	14	392	7.28	3890	1955	2000
DW/LOO	9-1/2	161	4.94	3225	1330	2000
PWI-30	11-7/8	280	6.18	4170	1705	2000
	9-1/4	181	4.81	2650	1280	2000
	9-1/2	193	4.94	2735	1330	2000
PWI-40	11-7/8	330	6.18	3545	1705	2000
	14	482	7.28	4270	1955	2000
	16	657	8.32	4950	2190	2000
	9-1/2	193	4.94	3345	1330	2000
	11-7/8	330	6.18	4315	1705	2000
PWI-45	14	486	7.28	5140	1955	2000
	16	665	8.32	5880	2190	2000
	7-7/8	133	4.10	2690	1000	2000
	9-1/2	207	4.94	3565	1330	2000
	11-7/8	345	6.18	4570	1705	2000
PWI-47	14	501	7.28	5420	1955	2000
	16	677	8.32	6185	2190	2000
	18	878	9.36	6500	2425	1450
	20	1112	10.40	7200	2660	1450
	9-1/2	186	4.94	3800	1330	2000
	11-7/8	322	6.18	4915	1705	2000
PWI-50	14	480	7.28	5860	1955	2000
	16	663	8.32	6715	2190	2000
	9-1/4	218	4.81	3665	1280	2000
	9-1/2	231	4.94	3780	1330	2000
PWI-60	11-7/8	396	6.18	4900	1705	2000
1 771 00	14	584	7.28	5895	1955	2000
	16	799	8.32	6835	2190	2000
	9-1/2	264	4.94	5195	1330	2000
	11-7/8	440	6.18	6730	1705	2000
	14	644	7.28	8030	1955	2000
PWI-70	16	873	8.32	9200	2190	2000
	18	1141	9.36	10355	2425	1450
	20	1447	10.40	11495	2660	1450
	9-1/2	261	6.08	5155	1430	2400
	11-7/8	442	7.60	6675	1925	2400
	14	648	8.96	7960	2125	2400
PWI-77	16	881	10.24	9120	2330	2400
PWI-77 PWI-77w	18	1152	10.24	10265	2535 2535	1800
1 VVI-7/VV						
	20	1463	12.80	11395	2740	1800
	22	1815	14.08	12520	2935	1300
	24	2209	15.36	13630	3060	1300

(See footnotes on next page)

Table 2a. Design Properties (Allowable Stress Design) for PWI Joists^(a) (continued)

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Joist Series	Joist Depth (in.)	EI ^(b) (10 ⁶ lbf-in. ²)	k ^(c) (10 ⁶ lbf)	M ^(d) (ft-lbf)	V ^(e) (lbf)	Uniform Vertical Load ^(f) (plf)
	9-1/2	392	6.08	7915	1430	2400
	11-7/8	661	7.60	10255	1925	2400
	14	965	8.96	12235	2125	2400
PWI-90	16	1306	10.24	14020	2330	2400
P VVI-90	18	1703	11.52	15780	2535	1800
	20	2155	12.80	17520	2740	1800
	22	2664	14.08	19245	2935	1300
	24	3232	15.36	20955	3060	1300

⁽a) The tabulated values are design values for normal duration of load. All values, except for EI, k and vertical load capacity shall be permitted to be adjusted for other load duration as permitted by the code.

Uniform Load:
$$\delta = \frac{5 \, w \ell^4}{384 \, EI} + \frac{w \ell^2}{k} \qquad \qquad [1]$$

Center-Point Load:
$$\delta = \frac{P\ell^3}{48EI} + \frac{2P\ell}{k}$$
 [2]

where δ = calculated deflection (in.),

P = concentrated load (lbf), El = bending stiffness of the l-joist (lbf-in.²), and

w = uniform load (lbf/in.),

 ℓ = design span (in.), k = coefficient of shear deflection (lbf).

⁽b) Bending stiffness.

⁽c) Coefficient of shear deflection. For calculating uniform load and center point load deflections of an I-joist in a simple-span application, use Equations 1 and 2.

⁽d) Moment capacity.

Shear capacity.

Blocking panel and rim joist uniform vertical load capacity (plf).

Table 2b. Design Properties (Allowable Stress Design) for PWI Joists

Able 2b. Design Properties (All Joist		ER ^(a) (I		IR ^(b) (lbf)		WS ^(c)	b _{EFF} (d)
Joist Series	Depth (in.)	No Web Stiffeners	With Web Stiffeners	No Web Stiffeners	With Web Stiffeners	Nails	(in.)
	7-7/8	$0.0 \times \ell_b + 900$	$0.0 \times \ell_b + 900$	$0.0 \times \ell_b + 1800$	$0.0 \times \ell_b + 1800$	4	
PWI-20	9-1/2	117.1 × ℓ _b + 710	0.0 × ℓ _b + 1120	142.9 × ℓ _b + 1490	$0.0 \times \ell_b + 2240$	4	1.62
1 W1 20	11-7/8	222.9 × ℓ _b + 525	$0.0 \times \ell_b + 1420$	$245.7 \times \ell_b + 1130$	211.4 × ℓ _b + 1535	4	1.02
	14	222.9 × ℓ _b + 525	$97.1 \times \ell_b + 1370$	$245.7 \times \ell_b + 1130$	$211.4 \times \ell_b + 1535$	4	
PWI-30	9-1/2	$77.7 \times \ell_b + 809$	$77.7 \times \ell_b + 809$	0.0 × ℓ _b + 1905	$0.0 \times \ell_b + 1905$	4	1.37
1 111 00	11-7/8	210.9 × l _b + 576	$210.9 \times \ell_b + 576$	$0.0 \times \ell_b + 1905$	$0.0 \times \ell_b + 1905$	4	1.07
	9-1/4	$0.0 \times \ell_b + 1080$	$0.0 \times \ell_b + 1080$	$0.0 \times \ell_b + 2160$	$0.0 \times \ell_b + 2160$	4	
	9-1/2	22.9 × ℓ _b + 1040	$0.0 \times \ell_b + 1120$	$0.0 \times \ell_b + 2240$	$0.0 \times \ell_b + 2240$	4	
PWI-40	11-7/8	194.3 $\times \ell_b + 740$	$0.0 \times \ell_b + 1420$	291.4 × ℓ _b + 1310	$0.0 \times \ell_b + 2840$	4	2.18
	14	$200.0 \times \ell_b + 730$	$0.0 \times \ell_b + 1710$	291.4 × ℓ _b + 1310	$205.7 \times \ell_b + 2120$	4	
	16	$200.0 \times \ell_b + 730$	$0.0 \times \ell_b + 1970$	291.4 × ℓ _b + 1310	$257.1 \times \ell_b + 2250$	8	
	9-1/2	$80.0 \times \ell_b + 840$	$0.0 \times \ell_b + 1120$	$0.0 \times \ell_b + 2240$	$0.0 \times \ell_b + 2240$	4	
D) / / / / / /	11-7/8	$245.7 \times \ell_b + 550$	$0.0 \times \ell_b + 1420$	180.0 × ℓ _b + 1620	137.1 × ℓ _b + 2120	4	4.00
PWI-45	14	$245.7 \times \ell_b + 550$	$80.0 \times \ell_b + 1430$	180.0 × ℓ _b + 1620	$240.0 \times \ell_b + 1760$	4	1.93
	16	$245.7 \times \ell_b + 550$	228.6 × l _b + 1170	$180.0 \times \ell_b + 1620$	$240.0 \times \ell_b + 1760$	8	
	7-7/8	$171.4 \times \ell_b + 565$	$14.3 \times \ell_b + 1085$	222.9 × l _b + 1030	168.6 × ℓ _b + 1535	4	1
	9-1/2	228.6 × l _b + 530	48.6 × ℓ _b + 1160	202.9 × ℓ _b + 1435	162.9 × ℓ _b + 1730	4	
	11-7/8	254.3 × ℓ _b + 515	17.1 × ℓ _b + 1410	254.3 × ℓ _b + 1260	$157.1 \times \ell_b + 2005$	4	2.18
PWI-47	14	$237.1 \times \ell_b + 575$	20.0 × ℓ _b + 1580	$300.0 \times \ell_b + 1105$	$151.4 \times \ell_b + 2250$	4	
	16	222.9 × l _b + 520	22.9 × ℓ _b + 1740	191.4 × ℓ _b + 1390	$145.7 \times \ell_b + 2485$	8	
	18	234.3 × l _b + 510	$22.9 \times \ell_b + 1905$	$182.9 \times \ell_b + 1480$	$140.0 \times \ell_b + 2720$	8	
	20	248.6 × ℓ _b + 495	$25.7 \times \ell_b + 2065$	177.1 × ℓ _b + 1560	134.3 × ℓ _b + 2955	10	
	9-1/2	46.9 × ℓ _b + 933	46.9 × ℓ _b + 933	$0.0 \times \ell_b + 2040$	$0.0 \times \ell_b + 2040$	4	
5 14// 5 2	11-7/8	$180.0 \times \ell_b + 700$	$180.0 \times \ell_b + 700$	$0.0 \times \ell_b + 2040$	$0.0 \times \ell_b + 2040$	4	
PWI-50	14	164.6 × ℓ _b + 727	213.7 × l _b + 641	$0.0 \times \ell_{\rm b} + 2040$	$0.0 \times \ell_{\rm b} + 2040$	4	1.62
	16	164.6 × l _b + 727	$293.7 \times \ell_b + 501$	$0.0 \times \ell_{\rm b} + 2040$	$0.0 \times \ell_{\rm b} + 2040$	8	
	9-1/4	$0.0 \times \ell_{b} + 1080$	$0.0 \times \ell_b + 1080$	$0.0 \times \ell_b + 2160$	$0.0 \times \ell_b + 2160$	4	
	9-1/2	22.9 × ℓ _b + 1040	$0.0 \times \ell_b + 1120$	$0.0 \times \ell_b + 2240$	$0.0 \times \ell_b + 2240$	4	
PWI-60	11-7/8	$194.3 \times \ell_b + 740$	$0.0 \times \ell_{\rm b} + 1420$	$291.4 \times \ell_b + 1310$	$0.0 \times \ell_{\rm b} + 2840$	4	2.18
00	14	$200.0 \times \ell_{\rm b} + 730$	$0.0 \times \ell_b + 1710$	$291.4 \times \ell_b + 1310$	$205.7 \times \ell_b + 2120$	4	
	16	$200.0 \times \ell_b + 730$	$0.0 \times \ell_b + 1970$	$291.4 \times \ell_b + 1310$	$257.1 \times \ell_b + 2250$	8	
	9-1/2	$0.0 \times \ell_b + 1120$	$0.0 \times \ell_b + 1120$	$0.0 \times \ell_b + 2240$	$0.0 \times \ell_b + 2240$	4	
	11-7/8	$148.6 \times \ell_b + 900$	$0.0 \times \ell_b + 1120$ $0.0 \times \ell_b + 1420$	$217.1 \times \ell_b + 1700$	$0.0 \times \ell_b + 2840$	4	
	14	$260.0 \times \ell_b + 705$	$67.4 \times \ell_b + 1474$	$308.6 \times \ell_b + 1380$	$154.3 \times \ell_b + 2610$	4	
PWI-70	16	$260.0 \times \ell_b + 705$	$216.0 \times \ell_b + 1214$	$308.6 \times \ell_b + 1380$	$257.1 \times \ell_b + 2250$	8	2.18
	18	$260.0 \times l_b + 705$ $260.0 \times l_b + 705$	$246.3 \times \ell_b + 1377$	$308.6 \times \ell_b + 1380$	$342.9 \times \ell_b + 2300$	8	
	20	-				10	
	20	$260.0 \times \ell_b + 705$	$260.0 \times \ell_b + 1353$	$308.6 \times \ell_b + 1380$	$342.9 \times \ell_b + 2300$	10	

(See footnotes on next page)

Table 2b. Design Properties (Allowable Stress Design) for PWI Joists (continued)

	Joist	ER ^(a) (lbf)		IR ^(b) (lbf)		WS ^(c)	b _{EFF} (d)
Joist Series	Depth (in.)	No Web Stiffeners	With Web Stiffeners	No Web Stiffeners	With Web Stiffeners	Nails	(in.)
	9-1/2	82.9 $\times \ell_b$ + 1140	$0.0 \times \ell_b + 1430$	94.3 × ℓ_b + 2365	$0.0 \times \ell_b + 2860$	4	
	11-7/8	$271.4 \times \ell_b + 810$	20.0 × ℓ _b + 1855	260.0 × ℓ _b + 1785	345.7 × ℓ _b + 1820	4	
	14	$271.4 \times \ell_b + 810$	134.3 × ℓ _b + 1655	260.0 × ℓ _b + 1785	$345.7 \times \ell_b + 1820$	4	
PWI-77	16	$271.4 \times \ell_b + 810$	$251.4 \times \ell_b + 1450$	260.0 × ℓ _b + 1785	$345.7 \times \ell_b + 1820$	8	2.18
PWI-77w	18	$271.4 \times \ell_b + 810$	$225.7 \times \ell_b + 1745$	260.0 × ℓ _b + 1785	$194.3 \times \ell_b + 3090$	8	2.10
	20	$271.4 \times \ell_b + 810$	291.4 × l _b + 1630	260.0 × ℓ _b + 1785	$194.3 \times \ell_b + 3090$	10	
	22	NA	291.4 × ℓ _b + 1880	NA	171.4 × ℓ _b + 3525	10	
	24	NA	291.4 × ℓ _b + 1880	NA	171.4 × ℓ _b + 3525	10	
	9-1/2	17.1 × ℓ_b + 1370	$0.0 \times \ell_b + 1430$	$0.0 \times \ell_b + 2860$	$0.0 \times \ell_b + 2860$	4	
	11-7/8	$285.7 \times \ell_b + 900$	$14.3 \times \ell_b + 1875$	$282.9 \times \ell_b + 2365$	$0.0 \times \ell_b + 3850$	4	
	14	$285.7 \times \ell_b + 900$	128.6 × ℓ _b + 1675	351.4 × ℓ _b + 2125	$225.7 \times \ell_b + 3065$	4	
PWI-90	16	$285.7 \times \ell_b + 900$	$245.7 \times \ell_b + 1470$	351.4 × ℓ _b + 2125	351.4 × ℓ _b + 2625	8	3.37
F VVI-90	18	$285.7 \times \ell_b + 900$	220.0 × ℓ _b + 1765	351.4 × ℓ _b + 2125	351.4 × ℓ _b + 3125	8	3.31
	20	$285.7 \times \ell_b + 900$	285.7 × ℓ _b + 1650	351.4 × ℓ _b + 2125	351.4 × ℓ _b + 3125	10	
	22	$142.9 \times \ell_b + 1050$	$285.7 \times \ell_b + 1900$	231.4 × ℓ _b + 2320	$351.4 \times \ell_b + 3375$	10	
	24	$142.9 \times \ell_b + 1050$	$285.7 \times \ell_b + 1900$	231.4 × ℓ _b + 2320	$351.4 \times \ell_b + 3375$	10	

⁽a) End reaction capacity for 1-3/4 inches ≤ ℓ_b ≤ 3-1/2 inches, where ℓ_b is the bearing length in inches. ER shall not exceed V as listed in Table 2a. See also Footnote (d).

⁽b) Intermediate reaction capacity for 3-1/2 inches ≤ ℓ_b ≤ 5-1/4 inches, where ℓ_b is the bearing length in inches. IR shall not exceed 2V, where V is listed in Table 2a. See also Footnote (d).

⁽c) Number of nails needed for web stiffeners, refer to Table 3 for web stiffener and nail dimensions.

After adjustment for pertinent load duration, ER shall not exceed $b_{EFF} \times \ell_b \times F_{c\perp}$ and IR shall not exceed $b_{EFF} \times \ell_b \times F_{c\perp} \times C_b$, where b_{EFF} is the effective width of the flange in inches, ℓ_b is the bearing length in inches, $F_{c\perp}$ is the reference compression design value perpendicular to grain in pounds per square inch and $C_b = (\ell_b + 0.375) \div \ell_b$. For the LVL flanges, $F_{c\perp} = 650$ psi. Do not adjust $F_{c\perp}$ for load duration when using the equation provided in this footnote. Compression of the support surface must also be checked.

Table 3.	Minimum Dimensions for Web Stiffeners	and Accompanying Nails ^(a)

Florge Width	Minimum Dimensions (in.)				
Flange Width	Web St	Neile			
(in.)	Thickness	Width	Nails		
1-1/2	15/32	2-5/16	2-1/2 x 0.131		
1-3/4	19/32	2-5/16	2-1/2 x 0.131		
2-1/16	23/32	2-5/16	2-1/2 x 0.131		
2-5/16	23/32	2-5/16	2-1/2 x 0.131		
2-1/2	23/32	2-5/16	2-1/2 x 0.131		
3-1/2	1-1/2	3-1/2	3-1/4 x 0.131		

⁽a) Web stiffener length is approximately 1/8 inch less than the clear distance between flanges.

APA – The Engineered Wood Association is an approved national standards developer accredited by American National Standards Institute (ANSI). APA publishes ANSI standards and Voluntary Product Standards for wood structural panels and engineered wood products. APA is an accredited certification body under ISO/IEC 17065 by Standards Council of Canada (SCC), an accredited inspection agency under ISO/IEC 17020 by International Code Council (ICC) International Accreditation Service (IAS), and an accredited testing organization under ISO/IEC 17025 by IAS. APA is also an approved Product Certification Agency, Testing Laboratory, Quality Assurance Entity, Validation Entity, and Product Evaluation Entity by the State of Florida, and an approved testing laboratory by City of Los Angeles.

APA - THE ENGINEERED WOOD ASSOCIATION

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